

D.M. Wills Associates Limited

Partners in Engineering, Planning and Environmental Services Peterborough

March 2022

Prepared for: Sunnyside Village Inc.

Functional Servicing Report

Mixed-Use Development 540 King Street East Town of Cobourg

D.M. Wills Project No. 19-10927





Summary of Revisions

Revision No.	Revision Title	Date of Release	Summary of Revisions
0	First Submission	March 29, 2022	

This report has been formatted considering the requirements of the Accessibility for Ontarians with Disabilities Act (AODA).



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1.0 Introduction

D.M. Wills Associates Limited (Wills) has been retained by Sunnyside Village Inc. to prepare a Functional Servicing Report (FSR) of the property located at 540 King Street East (Site), in the Town of Cobourg (Town). The FSR has been prepared in support of a rezoning, draft plan of condominium, and draft plan of subdivision application to be submitted by the planning consultant, FOTENN. The FSR provides preliminary servicing discussion and designs for the Site based on the concept plan provided by FOTENN for the application.

1.1 Site Location / Study Area

The Site is legally described as Part of Lot 10, Concession A; Described as Part 1, Plan 39R-374; Designated as PIN 51102-0224. The Site has 193 m of frontage on King Street East (County Road 2) and is bordered by an unopened road allowance to the west, the Canadian Pacific Railway to the north, existing rural residential to the east and future commercial land to the south. The Site location is shown in **Figure 1**.

The Site is a 3.97 hectare agricultural property, which currently includes a single residential dwelling with barns and associated silos, fields and vegetated areas.

The Town's Transportation Master Plan (August 2011) identifies the future construction of a new road and grade separation crossing over the Canadian Pacific / Canadian National (CN/CP) rail corridor within the unopened road allowance on the west of the Site. The proposed concept plan includes space to accommodate the grade separation.

The proposed development is a medium density residential development with 90 total residential units, approximately 1,400 m² of commercial space and community areas to create a mixed-use community within the Town. The proposed site includes:

- 7 detached homes, including the existing farm house which is to be preserved
- 4 semi-detached homes
- 55 townhouses
- 24 apartment style units above commercial space in the mixed-use blocks
- Larger barn to be preserved and restored as a commercial community amenity area

The proposed development also includes a private road and laneways that will be owned by the future condominium corporation. The concept plan provided by FOTENN is included in **Appendix A**.





Figure 1 - Site Location

1.2 Sourced Materials

This report relies on information provided from the sources listed below in Table 1.

Provider	Date	Reference #	Documentation
FOTENN	2022-02-22	-	Concept Plan
DFP Surveyors	2020-02-14	P2020-001	Legal and Topographic Survey
Town of Cobourg	1989-10	7294	King Street As-Constructed Drawings
Lakeshore Utilities	2020-06-22	-	GIS Water Map Markup
Pinchin Ltd.	2021-08-23	275057	Environmental Impact Study
GHD	2020-05-26	11211226	Geotechnical Investigation
GHD*	2019-04-24	11192099- ODA1	Overall Drainage Plan, King St East Sewer Analysis
Engage*	2019-12-13	10841	Post-Development Storm Drainage Areas, East Village Subdivision – Phase 5

Table 1 - Sourced Material Information

* Drainage figures were obtained from the Town's public website and are for reference purposes only.

2.0 Site Grading

2.1 Existing

The Site has generally level topography with an existing high point that splits the Site and directs approximately 30% of the drainage area east and the rest is directed to the west. The high point has an approximate elevation of 89.50 masl. The west area collects in a low-lying area along the property line, with an approximate low point elevation of 88.20 masl. The low-lying area spills west along the CP/CN rail corridor to a drainage feature through the East Village Subdivision (Phase 5). The drainage feature ultimately outlets to the King Street storm sewer.

Topographic survey and existing drainage figures are included in **Appendix A**.

2.2 Proposed

The proposed development is graded to direct drainage to an internal storm sewer network and stormwater management facilities. Proposed grading includes a safety/noise berm along the west boundary of the Site to meet the requirements of CP and CN rail.

The Site will require imported fill to bring proposed grades up to suitable elevations for gravity sewers and separation from groundwater, the estimated volume of fill is in the order of 20,000 m³. The grading will match into original ground at the buffer of the low-lying area as recommended in the Environmental Impact Study.

The internal road network will convey overland flow to King Street and the west outlet to provide safe access (maximum 0.3 m of ponding) within the Site.

3.0 Site Servicing

3.1 Sanitary

3.1.1 Existing

There is existing sanitary sewer infrastructure within the King Street East right-of-way (ROW); the infrastructure includes a 250 mm diameter sewer main under the eastbound lane of King Street along the frontage of the Site, and a 375 mm diameter stub at the southwest corner of the Site. The 250 mm sewer services approximately five properties east of Willmott Street. The 375 mm stub is the upstream end of the Willmott Street trunk sewer. The trunk sewer is assumed to be sized for future developments within the Town's settlement area including 540 King Street East and the Cobourg East Community Secondary Plan as shown on Schedule A of the Town's Official Plan.

The existing property has a 150 mm diameter sanitary service that connects to the 250 mm King Street East sewer main that will be removed as part of the development.

King Street As-constructed drawings are included in **Appendix A**.

3.1.2 Proposed

The proposed development will include an internal sanitary sewer network in accordance with the Town's Engineering Design Guidelines (April 2015).

Sanitary mains in the development will be 200 mm diameter and will connect to the trunk sewer at King Street and Willmott Street. Individual services will be 150 mm diameter minimum and connect to the on-site sanitary mains. Proposed design sewage flows for the development are 4.78 L/s.

The proposed servicing is shown in **Appendix B**, sanitary design flows are included in **Appendix C**.



3.2 Storm Water

3.2.1 Existing

There is existing storm sewer infrastructure in the King Street East ROW that is divided by a high point similar to the one that exists on the Site.

Approximately 140 m east of Wilmott Street, a 375 mm diameter storm sewer drains easterly and ultimately outlets from 600 mm diameter storm sewer into a drainage course at the southwest corner of King Street and Normar Road.

Approximately 55 m west of the Site, a 300 mm diameter sewer drains westerly and outlets into the King Street East / Coverdale Avenue trunk storm sewer. There are no existing storm sewer connections for this property.

King Street As-constructed drawings are included in **Appendix A**.

3.2.2 Proposed

The existing storm sewer in the King Street East ROW west of the Site will need to be extended approximately 55 m to the proposed development to provide an outlet for proposed catchment area 101.

A new structure is proposed on the existing storm sewer system to the east of the Site to provide an outlet for proposed catchment area 200.

Storm water from the Site will be controlled to pre-development flows before it outlets to the municipal storm infrastructure.

Refer to the Preliminary Stormwater Management report, prepared by Wills, dated February 2022, for details on the proposed SWM strategy for the development.

The proposed servicing is shown in **Appendix B**.

3.3 Water

3.3.1 Existing Conditions

Municipal water in is available for the proposed development from a 300 mm ductile iron water main in the King Street East ROW. The water main is stubbed in the boulevard on the north side of King Street at the unopened road allowance.

The existing farmhouse on the Site is serviced by a 50 mm polyethylene water service tapped from the 300 mm stub. The 50 mm service will be removed as part of the development.

King Street As-constructed drawings are included in **Appendix A**.



3.3.2 Proposed Conditions

3.3.2.1 Fire Flow

The Fire Underwriters Survey, Water Supply for Public Fire Protection (1999) guideline was referenced to calculate the required fire flows to the proposed development. The preliminary fire flow calculations are provided in **Appendix C – Water Servicing**.

For the purpose of this development, the fire flow requirement is calculated based on the worst-case scenario: largest building massing with the smallest building separations (Stacked Towns Block R).

The calculated worst-case fire flow requirement for the proposed development is 15,000 L/min, rounded off to nearest 1,000 L/min from 14,945L/min (249.09 L/s).

3.3.2.2 Domestic Demand

The domestic water demand has been calculated based on the Ministry of Environment Design Guidelines for Drinking Water Systems (2008) and is summarized in **Table 2**.

Types	No. Units	Maximum Daily Demand (L/s)	Peak Hour Demand (L/s)
Singles and Semi- detached	11	0.67	1.00
Townhouses	55	2.76	4.15
Mixed Use (Apartments)	24	0.73	1.09
Sub-Total	90	4.16	6.24
Mixes Use (Commercial)	N/A	0.09	0.14
Total		4.25	6.38

The Mixed Use Blocks and the Mixed Use Barn Structure are anticipated to include commercial uses such as; retail, office, meeting spaces, and amenity areas, that make up a small portion of the development. Total floor area for these uses is approximately 1,830 m².

The domestic demand calculations are provided in **Appendix C**.

3.3.3 Proposed Servicing

Watermains will be sized based on the greater of the following flow scenarios:

- Max Day Demand + Fire Flow: 4.25 + 249.09 = 253.34 L/s
- Peak Hour Demand = 6.38 L/s



Max day plus Fire Flow, 253.34 L/s, is the larger of the flow scenarios and will determine the size of watermains for the development.

The proposed 150 mm diameter watermain will be looped on-site to eliminate dead ends where possible. Fire hydrants will be installed at maximum spacing of 150 m, to provide for a maximum fire hose length of 75 m.

Watermain sizing will be modelled with Lakeshore Utilities Services Inc. (LUSI) during detailed design. Detailed design may consider reductions to fire flow requirements by including sprinkler systems, increased fire separations, and / or alternative non-combustible construction methods. Detailed design will also include any requirements for backflow prevention, pressure boosting and water metering, based on the existing watermain capacity and pressures.

The proposed servicing is shown in **Appendix B**.

4.0 Utilities

4.1 Lakefront Utilities Services Inc.

LUSI is the hydro provider for the Town and has confirmed the overhead line (threephase primary) in the King Street ROW can service the proposed development. Service details will be coordinated with LUSI during detailed design.

4.2 Gas

Enbridge Gas has confirmed natural gas is available in this area. There is an existing 4inch high pressure main on the south side of the King Street ROW. Sizing and service requirements such as pressure reducing stations will be determined by Enbridge Gas during detailed design.

4.3 Telecommunications

Bell has confirmed underground services are available within the King Street East ROW. The availability of fibre and possible service upgrades will be determined by Bell during detailed design.



5.0 Conclusions

The preceding Functional Servicing Report demonstrates the requirements of site servicing are met in accordance with the municipal and provincial Guidelines.

The site can be serviced as follows:

- Storm sewers for the minor storms will convey flows on-site in conjunction with Wills' Preliminary Stormwater Management Design and discharge to municipal infrastructure in the King Street right-of-way.
- Sanitary servicing will be provided through a connection to the 375mm diameter trunk main in Willmott Street.
- Water Servicing will be provided though a connection the 300mm diameter water main stub at King and Willmott Street and will include on-site looping.

If you require any further information, or have any questions, please do not hesitate to contact the undersigned.

Respectfully submitted,

Mark &

Mark Spiers, C.Tech. Municipal Project Manager

MS/dk/jh

Deborah Keay, P.Eng. Senior Project Engineer



6.0 Statement of Limitations

This report has been prepared by D.M. Wills Associates Limited on behalf of Sunnyside Village Inc. to address the requirements of the Town of Cobourg.

The conclusions and recommendations in this report are based on available background documentation and discussions with applicable agencies at the time of preparation.

The report is intended to determine the feasibility of the proposed development with respect to sanitary and water servicing of the subject lands. The design information provided in this report is preliminary in nature and is not to be used for construction purposes.

Any use that a third party makes of this report other than a functional servicing report for the proposed development is the responsibility of such third parties. D.M. Wills Associates Limited accepts no responsibility for damages, if any, suffered by a third party as a result of decisions made or action taken based on using this report for purposes other than a Functional Servicing Report for the property located at 540 King Street East, Cobourg.

Appendix A

Sourced Materials







SITE AREA Total Site

PARKING Residenti Residenti Mixed-Use Retail:

SETBACI Detachec Semi-Det Townhou Mixed-Use

DEVELOPMENT STATISTICS

RESIDEN Detached Semi-Det Townhou Mixed-Us TOTAL:

PARKING Residenti Visitor: Commerc Subtotal:

Mixed Use Total:

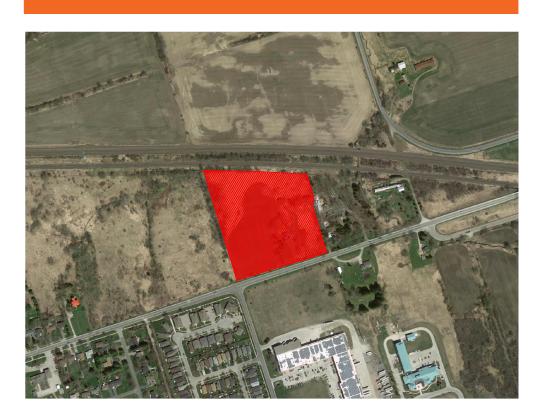
OPEN SP Environm Central Pa Barn Plaza

SITE INFORMATION

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ial (Townhouses):			1.0 p	o/unit	
se			1.0 p	o/unit	
		3 p/ ⁻	100m ²	GFA	
KS	F.Y.	C.Y.	S.Y.	R.Y.	
d	4.5m	2.4m	1.2m	7.5m	
tached	4.5m	2.4m	1.2m	7.5m	
JSES	4.5m	2.4m	1.2m	-	
se	1.8m	1.8	0m	-	

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tached:	4	
Jses:	55	
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	90	
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	22	33
cial:	36	38
	145	190
se Barn Structure:		49
		239
PACE		
nental Restoration	Zone	0.255ha
Park		0.136ha
za		0.085ha

Sunnyside Village 540 King St, Cobourg Concept Plan



LEGEND

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8	REVISED CONCEPT PLAN	2022.03.29	UMG
7	REVISED CONCEPT PLAN	2022.02.22	UMG
6	REVISED CONCEPT PLAN	2022.02.18	UMG
5	REVISED CONCEPT PLAN	2021.09.08	ΤK
4	PREFERRED CONCEPT PLAN	2021.06.18	ΤK
3	CONCEPT PLAN	2021.05.11	ΤK
2	CONCEPT PLAN	2021.04.23	ΤK
1	BASE PLAN	2021.04.07	ΤK
No.	REVISION	DATE	BY

CLIENT

SUNNYSIDE VILLAGE INC.

FOTENN Planning + Design

174 Spadina Avenue, Suite 304 Toronto, ON 616.789.4530 www.fotenn.com

DESIGNED TK/UMG REVIEWED UM

DATE 2022.03.29

SURVEYOR'S REAL PROPERTY REPORT

PART 1: PLAN OF SURVEY SHOWING TOPOGRAPHIC DETAIL OF PART OF LOT 10 CONCESSION A (GEOGRAPHIC TOWNSHIP OF HAMILTON) TOWN OF COBOURG COUNTY OF NORTHUMBERLAND



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(P1&M)

SIB(1142) 0.18W

36.14 (P1&S)

FRED PETRICH, B.Sc., O.L.S., O.L.I.P

20.12 (SET)

N87*16'30"W (M) (N86*55'35"E P1)

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20.12

21.13 (P1

PWF ON LINE

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-21.08 (M) +"

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IB(1219)

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METRIC

DISTANCES AND COORDINATES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048.

NOTES

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BEARINGS ARE UTM GRID DERIVED BY GNSS OBSERVATIONS AND REFERRED TO UTM ZONE 17 (81' WEST LONGITUDE), NAD83 (CSRS)(2010).

BEARINGS HAVE BEEN ROTATED COUNTERCLOCKWISE ON THE FOLLOWING PLANS TO ACCOUNT FOR CONVERSION TO UTM GRID BEARINGS: - P1, P2 BY 2°01'25"

ELEVATION NOTE

ELEVATIONS ARE GEODETIC, DERIVED BY RTK GPS OBSERVATIONS USING THE TOPNET REAL-TIME NETWORK SERVICE AND REFERRED TO CGVD28-1978 USING THE NRCAN HTv2.0 GEOID SEPARATION MODEL.

TEMPORARY BENCH MARK (TBM)

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PART 2: REPORT SUMMARY

DESCRIPTION: PART OF LOT 10, CONCESSION A; DESCRIBED AS PART 1, PLAN 39R-374; DESIGNATED AS PIN 51102-0224

REGISTERED EASEMENTS: NO EASEMENTS WERE RECORDED ON TITLE AT THE TIME OF THE SURVEY

COMPLIANCE WITH MUNICIPAL ZONING BY-LAWS: NOT CERTIFIED BY THIS REPORT

SIGNIFICANT OBSERVATIONS: NOTE THE POSITION AND TYPE OF FENCES AND BUILDINGS IN THE VICINITY OF THE PROPERTY LIMITS.

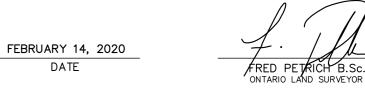
SURVEYOR'S CERTIFICATE

DATE

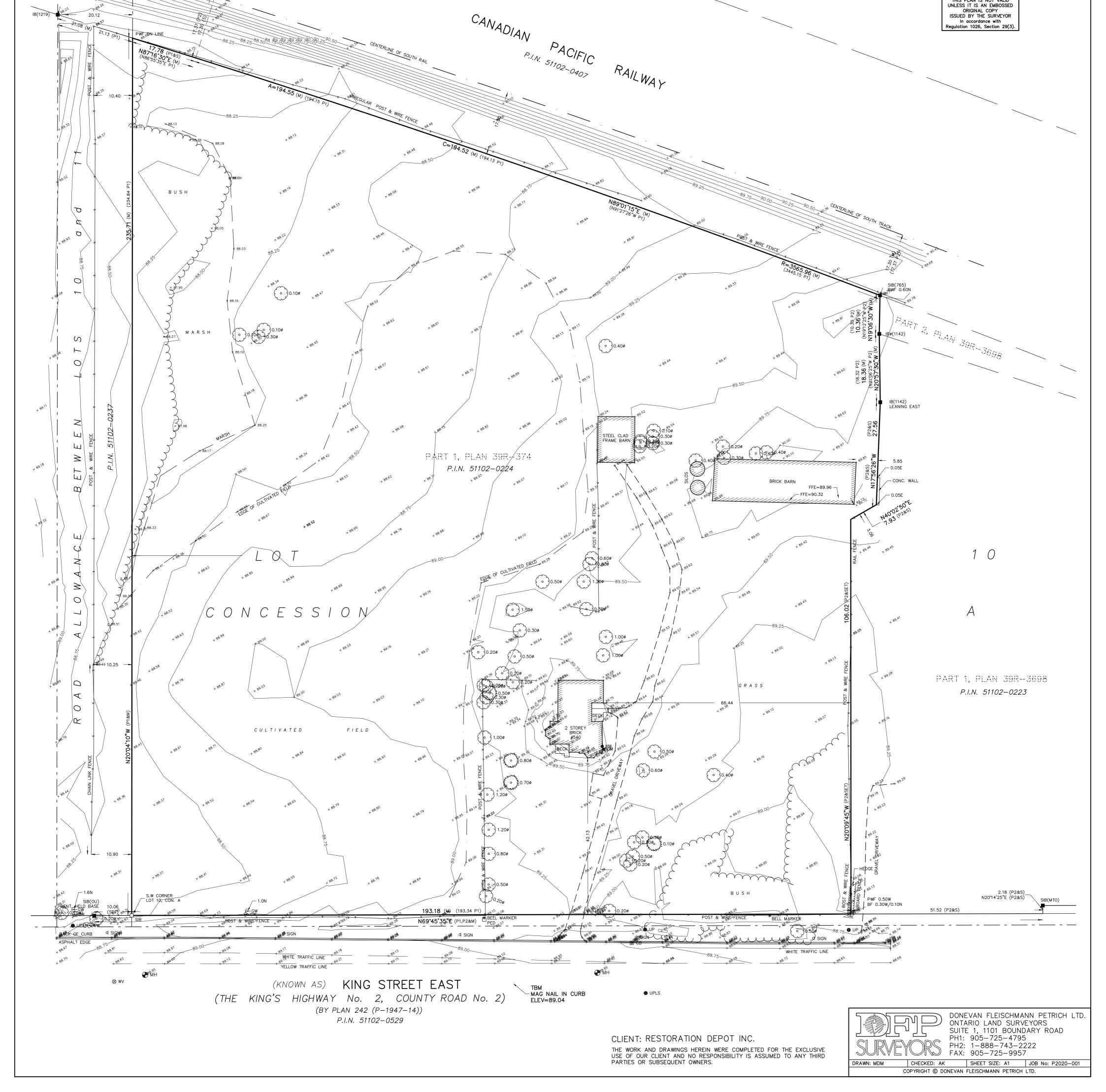
I CERTIFY THAT:

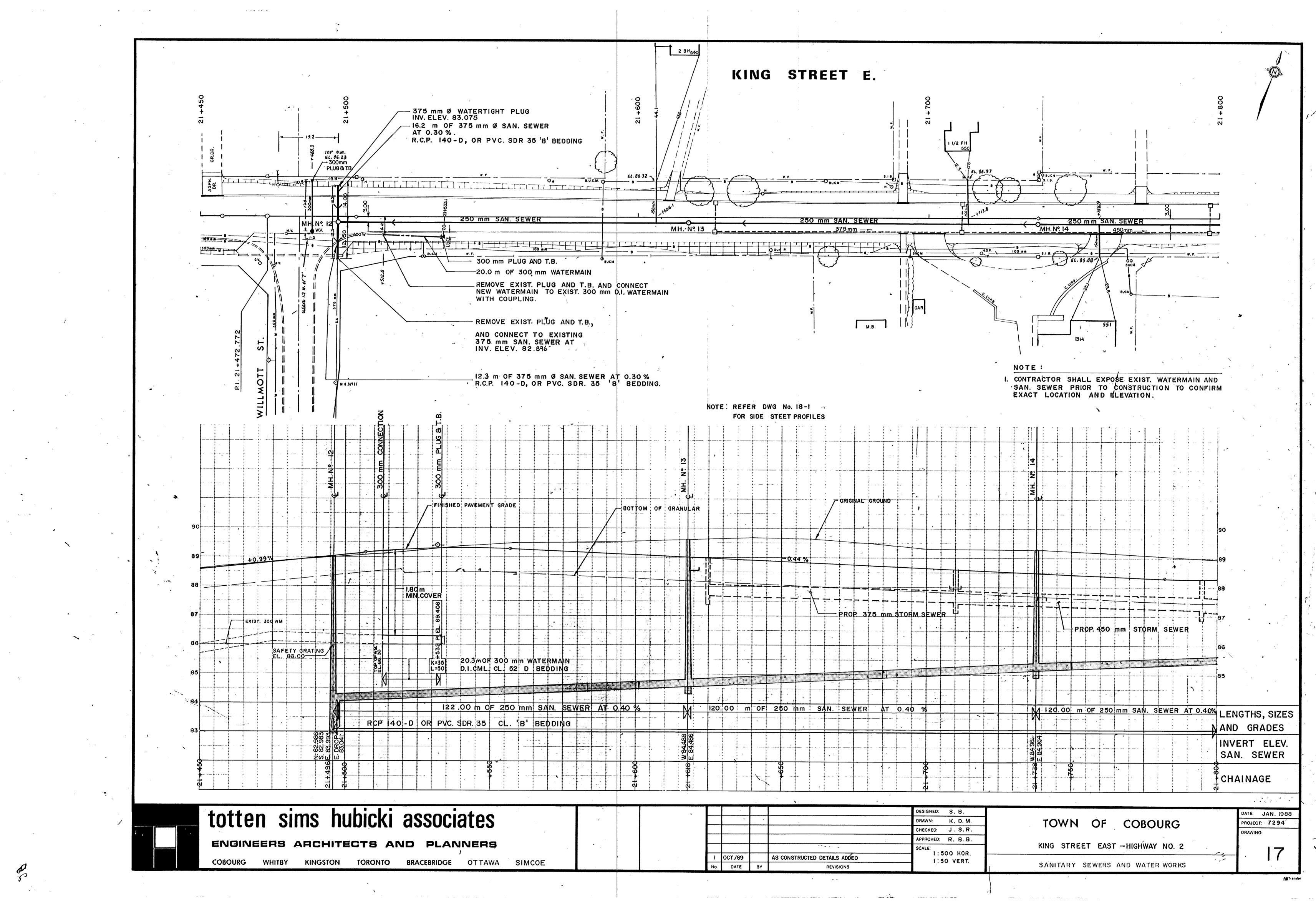
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2. THE SURVEY WAS COMPLETED ON THE 30th DAY OF JANUARY, 2020.

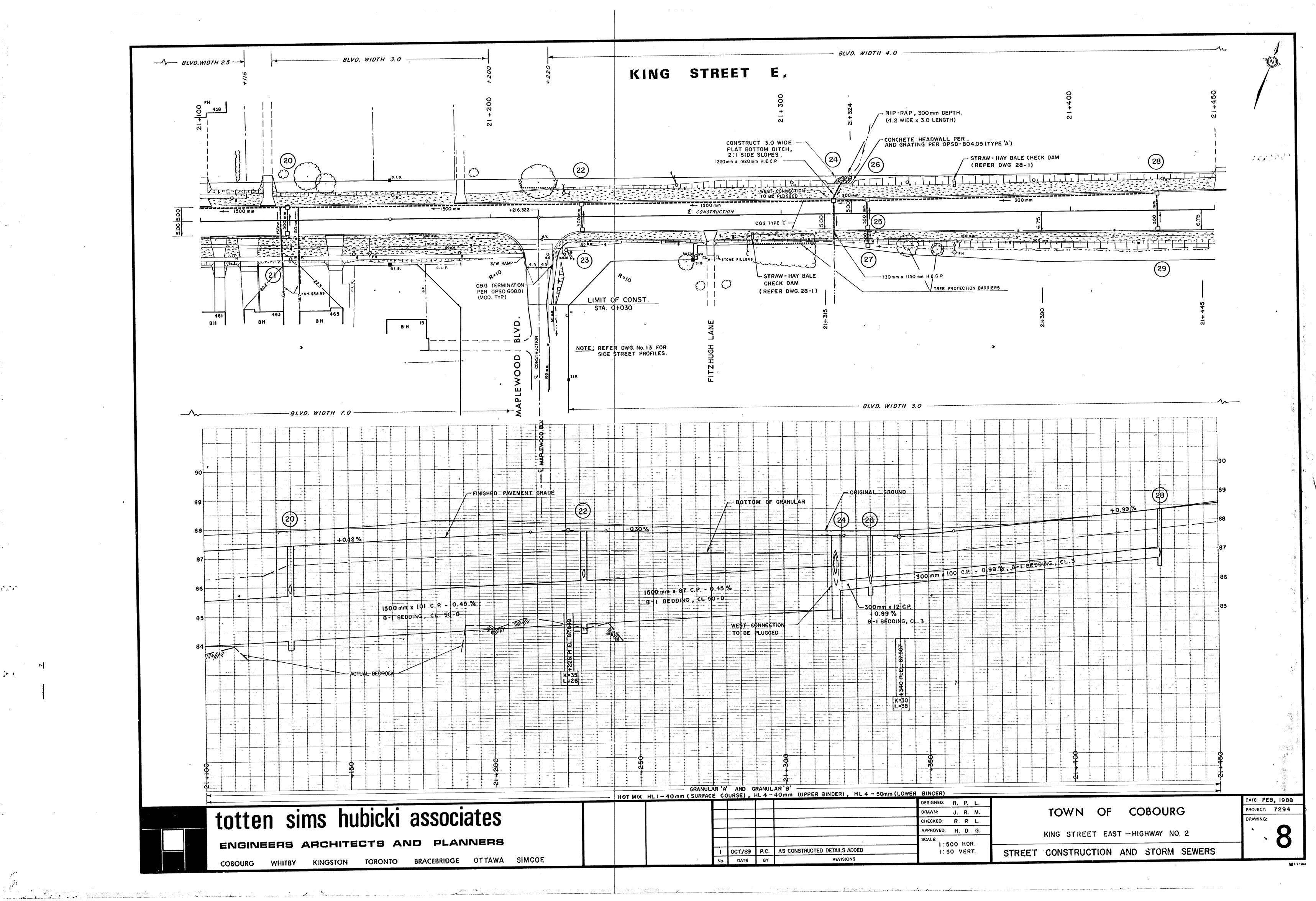






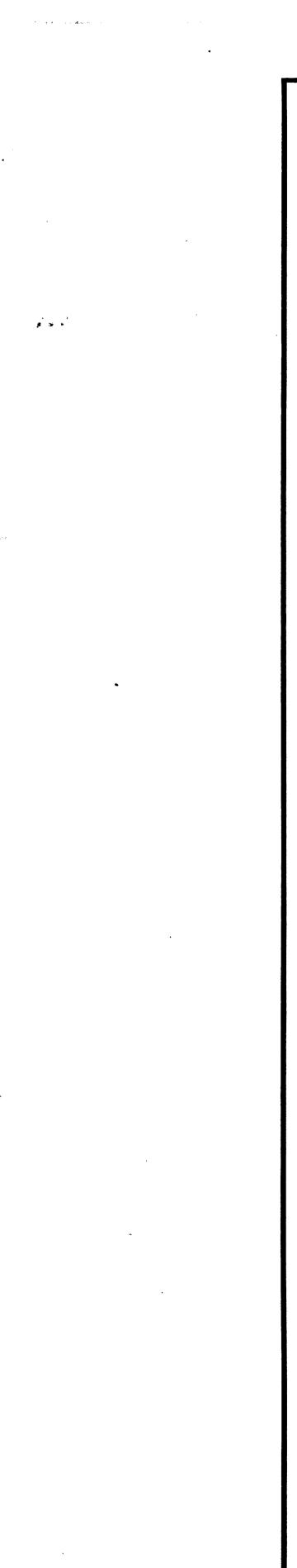


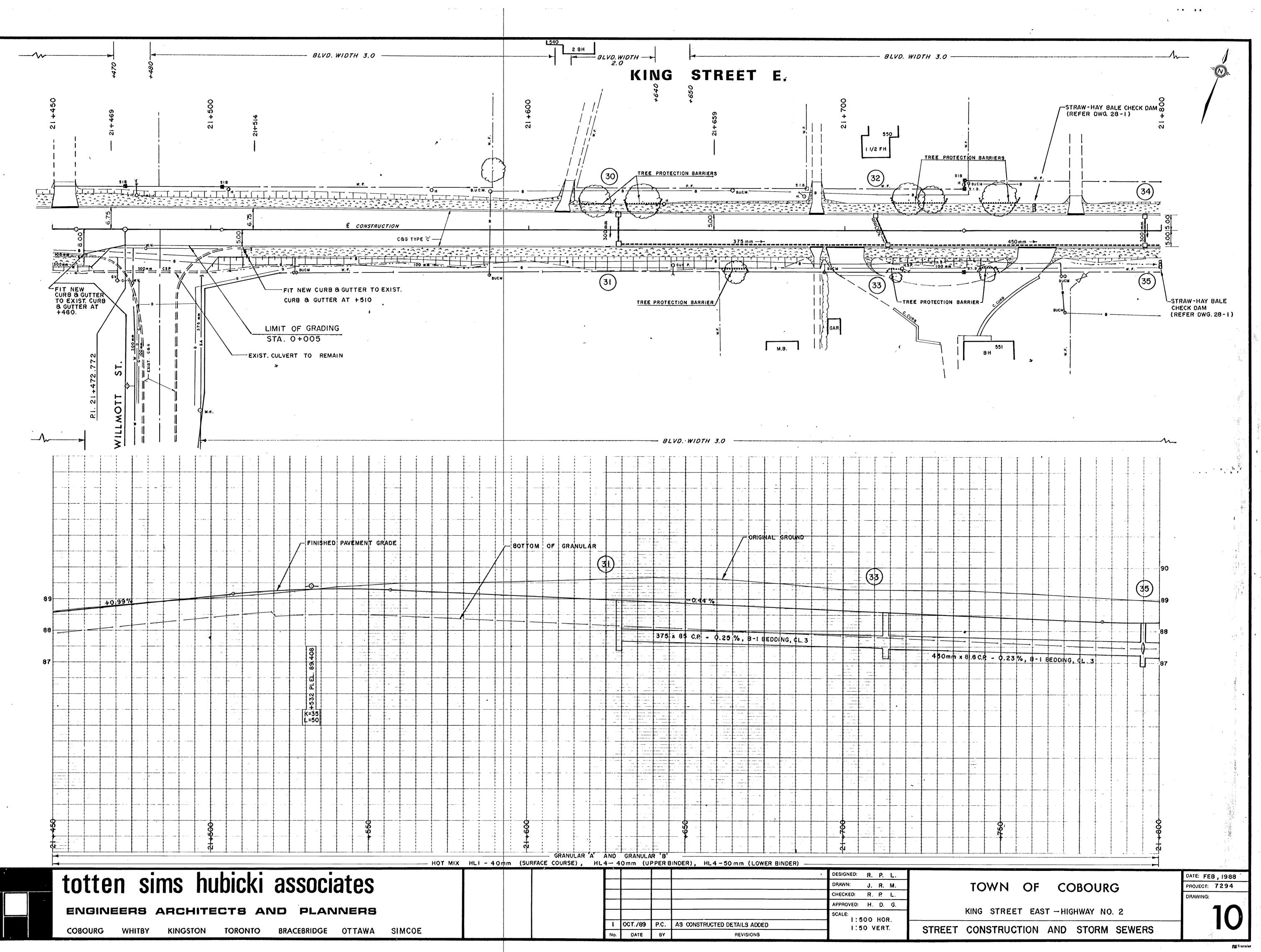
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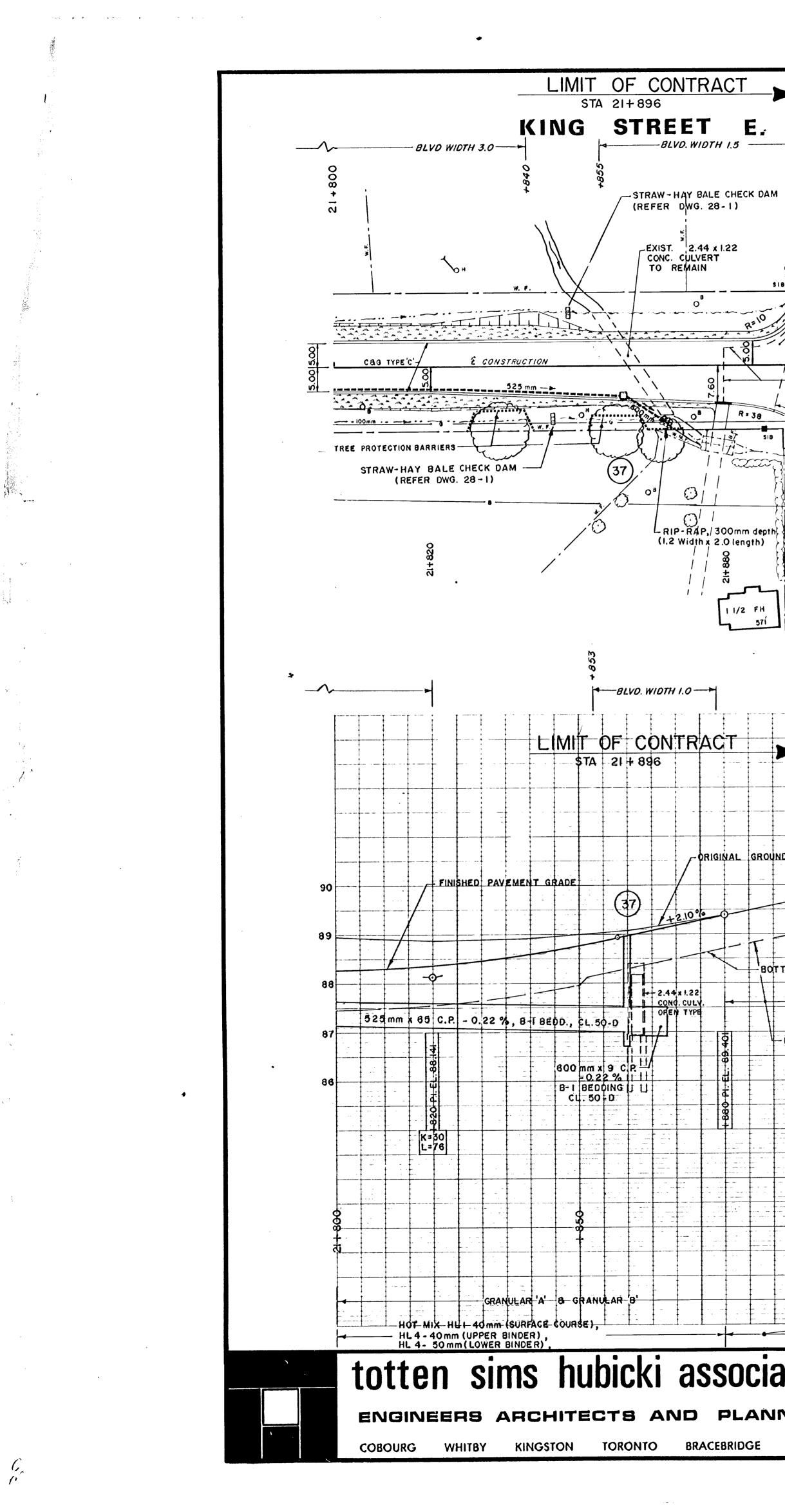
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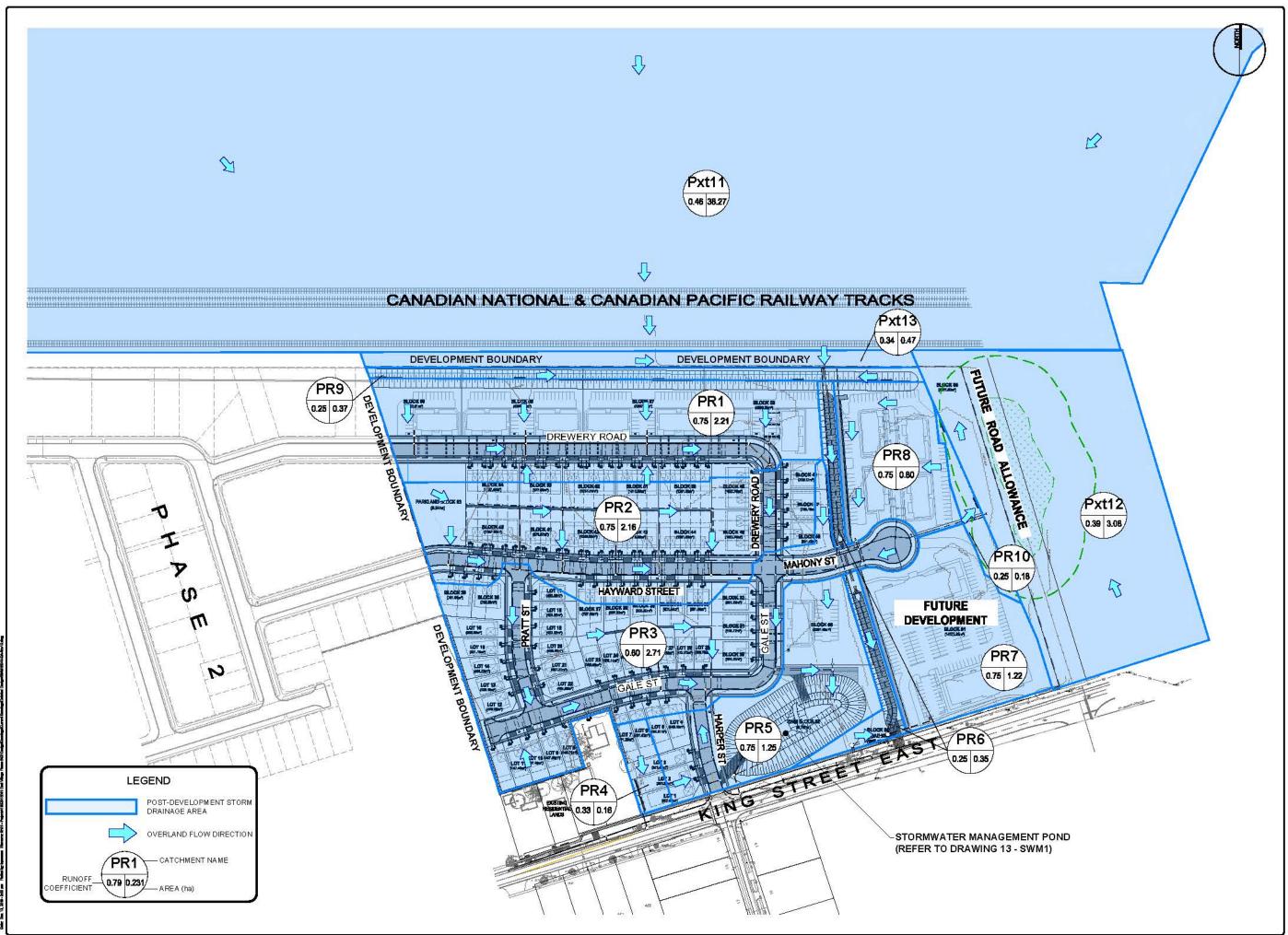




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SURVEY

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BENCHMARK

ELEVATIONS ARE GEODETIC AND REFERRED TO TOWN OF COBOURS BENCHMARK IS AND HAVING A GEODETIC REPATIONS OF

ELEV: 67.510m

NOTES:

- . ALL CONSTRUCTION AND MATERIALS TO BE IN ACCORDANCE WITH:
- TOWN OF COBOLING DESIGN STANDARDS
 ONTARIO PROVINCIAL STANDARD DRAWINGS |
- CHIMICS PROFILE CONTRACT DOCUMENTS AND ALL
- PECIFICATIONS REFERENCED HEREIN.
- THE CONTRACTOR BHALL CONSTRUCT ALL WORKIN ACCORDANCE WITH THE OCCUPATIONAL HEALTH AND BARETY ACT, HEALTH AND BARETY REGULATIONS FOR CONSTRUCTION PROJECTS.
- THE CONTRACTOR SHALL COORDINATE AND PAY FOR ALL PERMITS OR THERD PARTY WORKS ASSOCIATED WITH SERVICING CONNECTIONS.
- THE CONTRACTOR SHALL TAKE EXTREME GARE WHEN WORKING IN PRODUMITY TO EXISTING SERVICES.
- 5. THE CONTRACTOR SHALL RESTORE OR REPLACE DAMAGED DERVICES TO EXISTING OR BETER CONDITION.
- THE CONTRACTOR SHALL RESTORE ALL DETURBED AREAS TO EXISTING OR RETTER CONDITION, OR PER THE ENGINEERING AND LANDSCAPE SPECIFICATIONS REFERENCED INFERIO.
- THE CONTRACTOR SHALL COORDINATE AND PAY FOR ALL TRAFFIC CONTROL AND SAFETY MEASURED IN ACCORDANCE WITH THE ONTARIO TRAFFIC MANUAL, BOOK 7, TEMPORARY CONDITIONS.
- 8. THE CONTRACTOR SHALL DISPOSE OF ALL WASTE MATERIALS IN ACCORDANCE WITH THE MINISTRY OF THE ENVIRONMENT GLIDELINES AND LOCAL MUNICIPAL BYLANSI

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No.	REVISION	BY	DATE



EAST VILLAGE SUBDIVISION - PHASE 5

TO WIN OF COBOURO

POST-DEVELOPMENT STORM DRAINAGE AREAS

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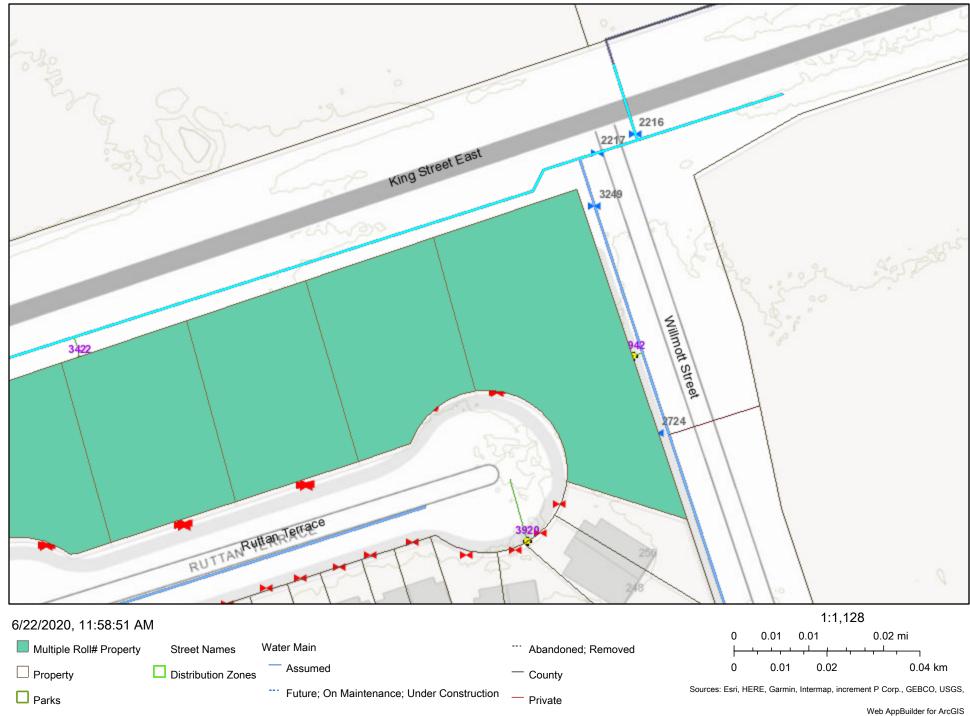
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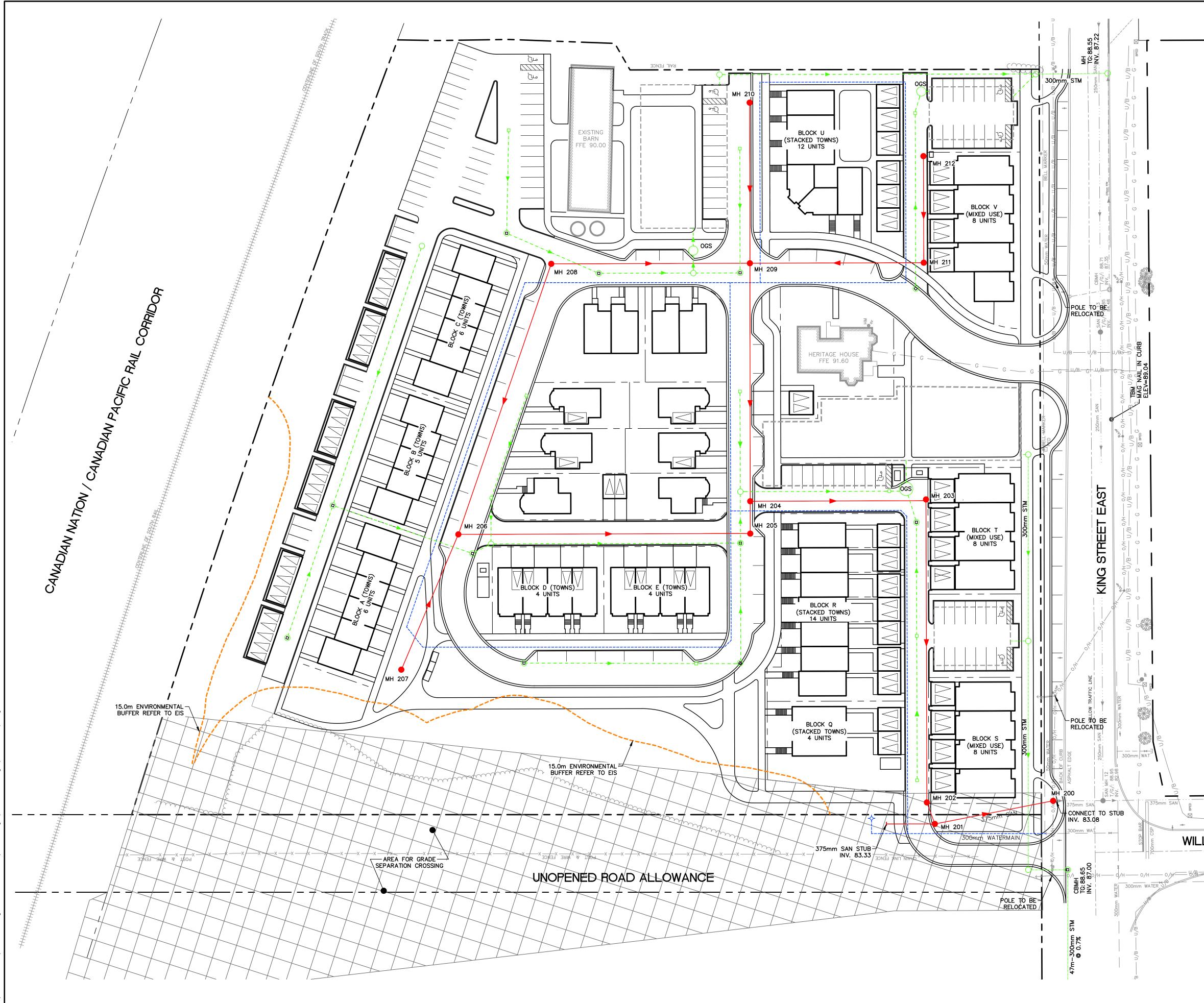
ArcGIS Web Map



Appendix B

Conceptual Servicing and Grading Plans

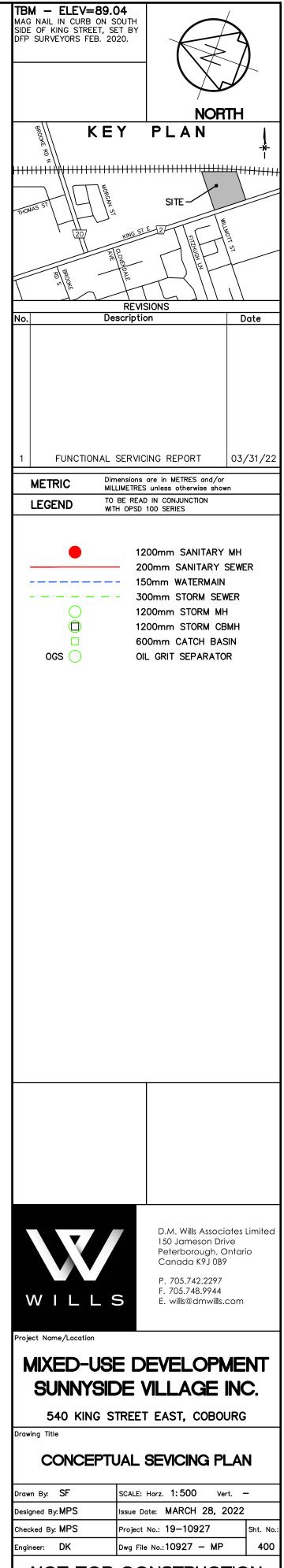




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MH ID	TOP OF GRATE	INVERT IN ELEVATION	INVERT OUT ELEVATION	
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211	89.03	84.81	84.78	
210	89.40	85.19	85.16	
209	89.47	84.36	84.33	
208	89.77	85.29	85.26	
206	89.42	84.53	84.50	
207	88.17	85.28	85.25	
205	89.25	84.13	84.10	
204	89.19	84.03	84.00	
203	88.90	83.78	83.75	
202	89.02	83.35	83.32	
201	89.03	83.27	83.24	
200	89.94	83.11	83.08	
200 89.94 SANITAR JP STREAM DOWN MH STREAM MH 212 211		LENGTH(m)	SLOPE	
212	211	27.5	2.0%	
211	209	45.0	1.0%	
210	209	41.5	2.0%	
209	204	62.0	0.5%	
208	209	51.5	2.0%	
208	206	74.5	1.0%	
207	206	38.0	2.0%	
206	205	74.0	0.5%	
205	204	8.5	1.0%	
204	203	45.5	0.5%	
203	202	78.5	0.5%	
202	201	6.0	1.0%	
201	200	31.0	0.5%	
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		D.M. Wills Associ	ates Limited
		150 Jameson Dri	ve
		Peterborough, C Canada K9J 0B9	
		P. 705.742.2297 F. 705.748.9944	
	WILLS	E. wills@dmwills.	com
· _	Project News 4		
-	Project Name/Location		
		E DEVELOPN	
—	SUNNYSIE	DE VILLAGE	INC. I
	540 KING S	TREET EAST, COBO	JRG
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	Designed By: MPS	Issue Date: MARCH 28, 2	2022
	Checked By: MPS	Project No.: 19-10927	Sht. No.:
		Dwg File No.: 10927 - MP	
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		CONSTRUCT	rion I

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Appendix C

Sanitary and Water Design Sheets



SANITARY SEWER DESIGN SHEET

Population Density											
Population M	lethod Us	ed:	Total Units								
Low Density			3.23								
Medium Density			2.68	pers / unit							
High Density			1.62	unit							

Unit Flo	ow Rates	
Domestic		L/cap./d
Industrial	90,000	L/ha./d.
Commercial		L/ha./d.
Institutional	112,000	L/ha./d.
Infiltration	0.26	L/ha./s.

Design	Criteria	
Min. Velocity		
Max. Velocity	3.00	m/s
Min. Diameter	200	mm
Max Capacity	80	%

Peaking	g Factor	
Dom. (Min/Max)	1.5	3.80
Industrial	2.0	
Commercial	2.5	
Institutional	2.0	

	Project Information
D.M. Wills Project	King St. Residential
D.M. Wills Project No.	10927
Project Location	King St. Hwy 2
Municipality	Cobourg
Design Standards	Town of Cobourg (February 2015)
Designed by	S. Robinson
Checked by	M. Spiers
Date	February 28, 2022
Design/As-built	Design

Street Name	Manł	oles					Domestic					Ind	lustrial	ustrial Commercial Inst		Commercial Instit		onal Design Flow Pipe Data												Comment																																		
			Ar	ea	Туре	and No. o	f Units	Pop	ulation	Pe	eak	ŀ	Area	A	rea	A	Area E		Area E		Area E:		Area E		Area E		Area E		Area		Area		Area		Area 1		Area 1		Area		Area F		Area E		Area E:		Area Ex		aneous	Infilt.	Dom.	Ind.	Comm.	Inst.	Total	Туре	Length	Grade	Dia.	Capacity	Percent	Ve	elocity	
	From	То	Unit	Accum	Low	Medium			Accum.	Harmon	Factor	Unit	Accum	Unit	Accum	Unit	Accum	Unit	Accum	Q _{iNF}	Q _{DOM}	Q _{IND}	Q _{COM}							Q _C	Capacity	Full	Actual																															
			(ha)	(ha)	Density	Density	Density			(M)	(M)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(l/s)	(l/s)	(l/s)	(l/s)	(l/s)	(l/s)	(l/s)	(l/s)		(m)	(%)	(mm)	(l/s)	(%)	(m/s)	(m/s)																															
540 King St East	Site	Stub	3.97	3.97	11	55	24	222	222	4.13	3.80	0.00	0.00	0.18	0.18	0.00	0.00	0.00	0.00	1.08	3.55	0.00	0.15	0.00	4.78	PVC	50.2	0.5	200	23.4	20.4%	0.75																																
King St East	Stub	MH12	0.20	4.17	0	0	0	0	222	4.13	3.80	0.00	0.00	0.00	0.18	0.00	0.00	0.00	0.00	1.13	3.55	0.00	0.15	0.00	4.83	PVC	14.0	0.3	375	96.0	5.0%	0.87																																
King St East	MH15			0.20	3	0	0	10	10	4.42	3.80	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00		0.16			0.00			120.0		250	37.6	0.6%	0.77																																
	MH14	MH13	0.20	0.40	1	0	0	3	13	4.40	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		0.21	0.00	0.00	0.00		PVC	120.0	0.4	250	37.6	0.8%	0.77		1																														
King St East	MH13	MH12	0.20	0.60	1	0	0	3	16	4.39	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.16	0.26	0.00	0.00	0.00	0.41	PVC	122.0	0.4	250	37.6	1.1%	0.77																																
Willmott Street	MH12	D/S MH	0.20	4.97	0	0	0	0	238	4.12	3.80	0.00	0.00	0.00	0.18	0.00	0.00	0.00	0.00	1.34	3.81	0.00	0.15	0.00	5.29	PVC	12.5	0.7	375	146.7	3.6%	1.33																																
															v Calcula				ning's n V																																													

Input Column Formulated Column U/S End of Pipe Run D/S End of Pipe Run

Exceeds Max Capacity

 $Q = V \times A$ $V = (1/n) \times R^{2/3} \times S^{1/2}$
 Manning's n Values

 HDPE
 0.010

 PVC
 0.013

 Concrete
 0.013

 Clay
 0.014

 Brick
 0.015





D.M. Wills Associates Ltd. 150 Jameson Drive Peterborough, ON · K9J 0B9 Tel: (705) 742-2297 Fax: (705) 741-3568

	PROJECT INFORMATION				
PROJECT LOCATION:	Town of Cobourg				
DM WILLS PROJECT:	King Street Residential				
DM WILLS PROJECT No .:	10927 WILLS				
DESIGNED BY:	S.Robinson				
<u>CHECKED BY:</u>	M. Spiers <u>Date:</u> 02/22/22				
	PRELIMINARY FIRE FLOWS - BLOCK R				
<u>CRITERIA USED:</u>	Fire Underwriters Survey				
	Water Supply for Public Fire Protection				
CALCULATION 1:					
F = 220 x C x √A	F= The required fire flow in litres per minute				
	C = 1.5 for wood frame construction				
	1.0 for ordinary construction				
	0.8 for non-combustible construction				
	0.6 for fire-resistive construction				
	$A_{=}$ Total floor area in m (all above ground floors)				
	A= 2,195.00 m ²				
	F= 10307.2 L/min				
CALCULATION 2: -25%	Non-Combustible Rapid Burning +25%				
-25%	Non-CombustibleRapid Burning+25%Limited CombustibleFree Burning+15%				
-13% 0%	Combustible Free Burning +15%				
070					
	F= 10307.2 L/min				
CALCULATION 3:					
	Value from CALCULATION 2 can be reduced by up to				
	50% for sprinklering. No sprinklering is assumed at this				
	time.				
	F= 0.0 L/min Reduction				
CALCULATION 4:	Increase value from CALCULATION 2 by 45% for				
N = + 10%	exposed structure separations.				
E = + 5%					
S = + 10%	F= 4638.23 L/min Increase				
W = + 20%					
FINAL FIRE FLOW:					
	F= 14945.41 L/min				
	F = 249.09 L/s				

	PROJECT	INFORMATIC)N			
PROJECT LOCATION:	Town of Cobourg					
DM WILLS PROJECT:	King Street Reside	ntial			∇I	
DM WILLS PROJECT No.:	10927					
	6 Debinson			vv		
DESIGNED BY:	S. Robinson			Data	02/22/22	
<u>CHECKED BY:</u>	M. Spiers			Date:	02/22/22	
PROPOSED	Domestic water d)emand - Si	ngle and Semi De	etached		
<u>CRITERIA USED:</u>	Ministry of Environ	•				
	tor Drir	iking water	Systems (2008)			
DOMESTIC WATER DEMAI	<u>ND:</u>	450	L/person/day	1	А	
		11	units		С	
<u>No. OF UNITS:</u> No. PERSONS/UNIT*:		3.2			D	
MAX. DAY FACTOR**:		3.2 3.60	persons/unit		E	
PEAK HOUR FACTOR**:		5.00			F	
		0				
CALCULATIONS:						
Total Residential Develop (flow previously calculat FF=						
Average Day Demand:		Maximum Day Demand:				
$F_{AvgD} = A xC x D$		$F_{MaxD} = F_{AvgD} \times E$				
$F_{AvgD} = 15988.5 L/Day$		$F_{MaxD} = 0.67 L/s$				
$F_{AvgD} = 0.19 L/s$						
Peak Hour Demand:		Total Maximum Demand: (Max Day + Fire Flow vs. Peak Hour)				
F _{PeakH} =	$F_{\text{PeakH}} = F_{\text{AvgD}} \mathbf{x} \mathbf{F}$		$F_{Total} = F_{MaxD} + FF$			
$F_{PeakH} = 1.00 L/s$		$F_{Total} = 249.76 L/s$				
<u>Notes:</u> * Persons per unit assumed t D1.01, Design Flow for new r ** Max Day and Peak Hour Fa	esidential actors taken from MOE	n of Cobourg design Guide	Design Guidelines, S	ing factors	for Drinking-	

Water Systems serving fewer than 500 people. Assumed equivelant population of 300 people.

	PROJECT	INFORMATIC	DN				
PROJECT LOCATION:	Town of Cobourg						
DM WILLS PROJECT:	King Street Reside	ntial					
DM WILLS PROJECT No.:	10927			WILLS			
DESIGNED BY:	S. Robinson						
CHECKED BY:	M. Spiers			Date: 02/22/22			
<u></u>							
PRC	POSED DOMESTIC V	vater dema	AND - Townhouses				
<u>CRITERIA USED:</u>	Ministry of Environ	ment, Desig	n Guidelines				
	for Drir	nking Water	Systems (2008)				
Domestic water dema	ND:	450	L/person/day	А			
<u>No. OF UNITS:</u>		55	units	С			
No. PERSONS/UNIT*:		2.68	persons/unit	D			
MAX. DAY FACTOR**:		3.6		E			
PEAK HOUR FACTOR**:		5.4		F			
CALCULATIONS:							
Total Residential Develo (flow previously calculat FF=							
Average Day Demand:		Maximu	Maximum Day Demand:				
$F_{AvgD} = A xC x D$		F _{Ma}	$F_{MaxD} = F_{AvgD} \times E$				
F _{AvgD} = 66330.0 L/Day		$F_{MaxD} = 2.76 L/s$					
F _{AvgD} =	= 0.77 L/s						
Peak Hour Demand:		Total Maximum Demand: (Max Day + Fire Flow vs. Peak Hour)					
F _{PeakH} =	= F _{AvgD} x F	$F_{Total} = F_{MaxD} + FF$					
F _{PeakH} =	= 4.15 L/s	F _{Tot}	al = 251.85 L/s				
<u>Notes:</u> * Persons per unit assumed t D1.01, Design Flow for new r ** Max Day and Peak Hour F	esidential	-	-				

** Max Day and Peak Hour Factors taken from MOE design Guideline, Table 3-3: Peaking factors for Drinking-Water Systems serving fewer than 500 people. Assumed equivelant population of 300 people.

	Р	ROJECT INFORMATIC	DN			
PROJECT LOCATION:	Town of Col					
DM WILLS PROJECT:	King Street Residential					
DM WILLS PROJECT No.:	10927		5			
DESIGNED BY:	S.Robinson				- 0	
CHECKED BY:	M. Spiers			Date: 0	2/22/22	
ONEORED DT.				<u>Date:</u> 0		
PROPOSEI	D DOMESTIC V	/Ater Demand - I	Mixed Use (Apartr	ments)		
<u>CRITERIA USED:</u>	Ministry of F	nvironment, Desig	un Guidelines			
CRIERIA USED.	5	or Drinking Water				
		0	y	٨		
DOMESTIC WATER DEMA	<u>ND:</u>	450	L/person/day	A		
<u>No. OF UNITS:</u>		24	units	С		
No. PERSONS/UNIT*:		1.62	persons/unit	D		
MAX. DAY FACTOR**:		3.6		E		
PEAK HOUR FACTOR**:		5.4		F		
CALCULATIONS:						
Total Residential Develo (flow previously calculat FF=	ed)	N /s				
Average Day Demand:		Maximu	ım Day Demand:			
$F_{AvgD} = A xC x D$		F _{Ma} ,	$F_{MaxD} = F_{AvqD} x E$			
F _{AvgD} = 17496.0 L/Day		ay F _{Ma} ,	$F_{MaxD} = 0.73 L/s$			
F _{AvgD} =	= 0.20 L/s	5				
Peak Hour Demand:			Total Maximum Demand: (Max Day + Fire Flow vs. Peak Hour)			
FROAKH =	$F_{PeakH} = F_{AvgD} \times F$		$F_{Total} = F_{MaxD} + FF$			
FEANI	$F_{\text{PeakH}} = 1.09 \text{ L/s}$		$F_{Total} = 249.82 L/s$			
	= 1.09 L/s					
F _{PeakH} = <u>Notes:</u>		to Town of Cobourg	Design Guidelines, Se	ection		
F _{PeakH} =	o be equivelant	to Town of Cobourg	Design Guidelines, Se	ection		

Water Systems serving fewer than 500 people. Assumed equivelant population of 300 people.

	PROJECT IN	FORMATION	J		
PROJECT LOCATION:	Town of Cobourg				
DM WILLS PROJECT:	King Street Resident	ial			
DM WILLS PROJECT No.:	10927			WILLS	
DESIGNED BY:	S. Robinson				
CHECKED BY:	M. Spiers		 Dat	e: 02/22/22	
PRO	Posed domestic wa	TER DEMAN	ID - Commercial		
<u>Criteria Used:</u>	Ministry of Environment, Design Guidelines for Drinking Water Systems (2008)				
Domestic water demai	<u>ND*:</u>	28	m³/hectare/day	А	
<u>AREA:</u>		0.183		В	
MAX. DAY FACTOR**:		1.5		С	
PEAK HOUR FACTOR**:		2.4		D	
CALCULATIONS:					
Total Residential Develop (flow previously calculat FF=					
Average Day Demand:		Maximur	n Day Demand:		
F _{AvgD} =	A xB	F _{MaxD}	= F _{AvgD} x E		
F _{AvgD} =	= 5.12 m³/day	F _{MaxD}	= 0.09 L/s		
F _{AvgD} =	= 5124.00 L/Day				
F _{AvgD} =	= 0.06 L/s				
Peak Hour Demand: $F_{PeakH} = F_{AvgD} \times F$ $F_{PeakH} = 0.14 L/s$		Total Maximum Demand: (Max Day + Fire Flow vs. Peak Hour) $F_{Total} = F_{MaxD} + FF$ $F_{Total} = 249.18 L/s$			
Notes:					